Revisions and Errata List
AISC Steel Design Guide 29, 1st Edition, 1st printing (Printed Edition) and March 2015 Revision (Digital Edition)
August 20, 2020

The following list represents corrections made to the first printing (dated 2013) and the March 2015 revision (digital edition) of the first edition of AISC Design Guide 29, *Vertical Bracing Connections—Analysis and Design.*

Page(s)	Item

127 Under the heading "Tension rupture component," revise the text as follows:

$U_{bs} = 1$ from AISC *Specification* Section J4.3 because the bolts are uniformly loaded

$$A_{nt} = (0.435 \text{ in.})\left[7.96 \text{ in.} - 5.00 \text{ in.} - 1(1\text{⁹⁄₁₆} + \text{⅛ in.})\right]$$
$$= 0.80 \text{ in.}^2$$
$$U_{bs} F_u A_{nt} = 1(65 \text{ ksi})(0.80 \text{ in.}^2)$$
$$= 52.0 \text{ kips}$$

The available strength for the limit state of block shear rupture is:

$$0.60 F_u A_{nv} + U_{bs} F_u A_{nt} = 285 \text{ kips} + 52.0 \text{ kips}$$
$$= 337 \text{ kips}$$
$$0.60 F_y A_{gv} + U_{bs} F_u A_{nt} = 351 \text{ kips} + 52.0 \text{ kips}$$
$$= 403 \text{ kips}$$

128 Replace the first line of text and calculation box with the following:

Therefore, $R_n = 337$ kips.

LRFD	ASD
$\phi R_n = 0.75(2)(337 \text{ kips})$ $= 506 \text{ kips} > 270 \text{ kips}$ **o.k.**	$\frac{R_n}{\Omega} = 2\left(\frac{337 \text{ kips}}{2}\right)$ $= 337 \text{ kips} > 180 \text{ kips}$ **o.k.**

164 The calculation for β should be replaced with the following:

$$\beta = \frac{\alpha - e_b \tan\theta + e_c}{\tan\theta} \qquad \text{(from } \textit{Manual} \text{ Eq. 13-1)}$$

$$= \frac{16.3 - (8.85 \text{ in.})(1.33) + 0}{1.33}$$
$$= 3.41 \text{ in.}$$