

**Revisions and Errata List**  
**AISC Steel Design Guide 31, 1<sup>st</sup> Printing (Printed and Digital Editions)**  
**February 16, 2023**

The following list represents corrections to the first printing of AISC Design Guide 31, *Castellated and Cellular Beam Design*.

**Page(s) Item**

8 In Section 2.3, first sentence, revise Section 3.3 to Section 3.4.

18 Revise Equation 3-12 to:

$$T_o = M_r \left[ \frac{1 - \frac{(q)(X_i)}{T_1}}{d_{\text{effec-noncomp}}} \right]$$

19 In Section 3.4, second paragraph, third sentence, revise the end of the sentence as follows:

“...a function of  $2h/e$ ; the value of  $2h/e$  need not be taken as less than 2.”

20 Revise Equation 3-19 to:

$$V_{rh} = \left| \frac{M_{r(i+1)} - M_{r(i)}}{d_{\text{effect-noncomp}}} \right| = |T_{r(i)} - T_{r(i+1)}|$$

27 The value of  $y_o$  in Table 4-1 should be  $y_o = 0.568$  in.

32-33 The flexural torsional buckling check is revised as follows:

$$\bar{r}_o^2 = x_o^2 + y_o^2 + \frac{I_x + I_y}{A_g} \quad (\text{Spec. Eq. E4-9})$$

$$\begin{aligned} &= y_o^2 + \frac{I_x + I_y}{A_{ee}} \\ &= (0.568 \text{ in.})^2 + \frac{1.13 \text{ in.}^4 + 1.18 \text{ in.}^4}{1.45 \text{ in.}^2} \\ &= 1.92 \text{ in.}^2 \end{aligned}$$

$$\begin{aligned} F_{ez} &= \left[ \frac{\pi^2 (29,000 \text{ ksi})}{(3.00 \text{ in.})^2} + (11,200 \text{ ksi})(0.022 \text{ in.}^4) \right] \frac{1}{(1.45 \text{ in.}^2)(1.92 \text{ in.}^2)} \\ &= 11,500 \text{ ksi} \end{aligned}$$

$$H = 1 - \frac{x_o^2 + y_o^2}{\bar{r}_o^2} \quad (\text{Spec. Eq. E4-8})$$

$$= 1 - \frac{(0.568 \text{ in.})^2}{1.92 \text{ in.}^2}$$

$$= 0.832$$

$$F_e = \left[ \frac{25,800 \text{ ksi} + 11,500 \text{ ksi}}{2(0.832)} \right] \left[ 1 - \sqrt{1 - \frac{4(25,800 \text{ ksi})(11,500 \text{ ksi})(0.832)}{(25,800 \text{ ksi} + 11,500 \text{ ksi})^2}} \right]$$

$$= 10,300 \text{ ksi}$$

$$F_{cr} = \left( 0.658 \frac{F_y}{F_e} \right) F_y$$

$$= \left( 0.658 \frac{50 \text{ ksi}}{10,300 \text{ ksi}} \right) (50 \text{ ksi}) \quad (\text{Spec. Eq. E3-2})$$

$$= 49.9 \text{ ksi}$$

$$P_n = F_{cr} A_{tee}$$

$$= (49.9 \text{ ksi})(1.45 \text{ in.}^2)$$

$$= 72.4 \text{ kips}$$

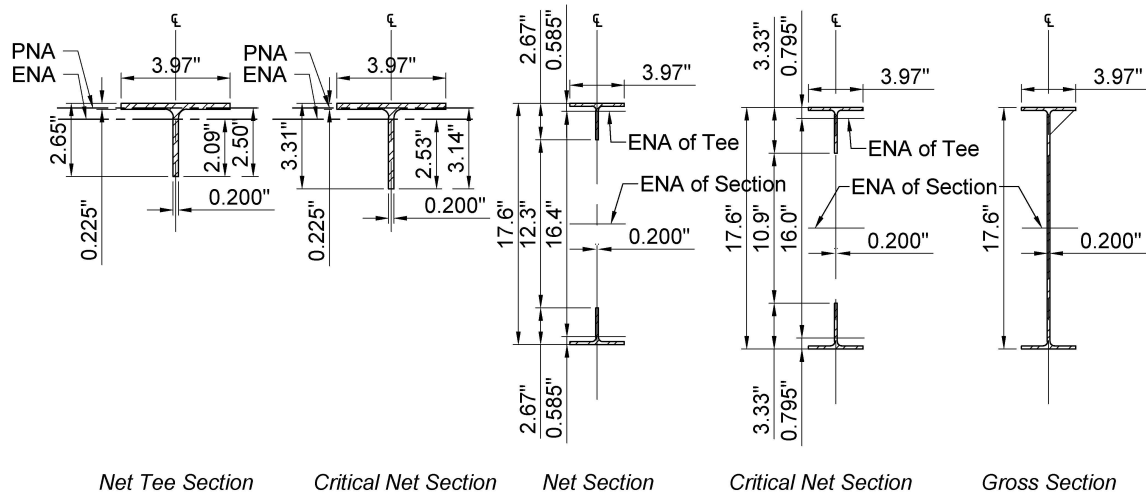
The available compressive strength of the tee is:

LRFD	ASD
From Table 4-3, $P_r = 47.8 \text{ kips}$ $P_u = \phi_c P_n$ $= 0.90(72.4 \text{ kips})$ $= 65.2 \text{ kips} > 47.8 \text{ kips} \quad \mathbf{o.k.}$	From Table 4-3, $P_r = 34.9 \text{ kips}$ $P_a = \frac{P_n}{\Omega_c}$ $= \frac{72.4 \text{ kips}}{1.67}$ $= 43.4 \text{ kips} > 34.9 \text{ kips} \quad \mathbf{o.k.}$

42 The value of  $y_o$  in Table 4-7 should be  $y_o = 0.448 \text{ in.}$

The value of  $y_o$  in Table 4-8 should be  $y_o = 0.668 \text{ in.}$

Revise the dimensions in Figure 4-4 to:



48-49 The flexural torsional buckling check is revised as follows:

$$\bar{r}_o^2 = x_o^2 + y_o^2 + \frac{I_x + I_y}{A_g} \quad (\text{Spec. Eq. E4-9})$$

$$\begin{aligned}
 &= y_o^2 + \frac{I_{x\text{-tee-crit}} + I_y}{A_{\text{tee-crit}}} \\
 &= (0.668 \text{ in.})^2 + \frac{1.52 \text{ in.}^4 + 1.18 \text{ in.}^4}{1.51 \text{ in.}^2} \\
 &= 2.23 \text{ in.}^2
 \end{aligned}$$

$$\begin{aligned}
 F_{ez} &= \left[ \frac{\pi^2 (29,000 \text{ ksi})}{(6.15 \text{ in.})^2} + (11,200 \text{ ksi})(0.023 \text{ in.}^4) \right] \frac{1}{(1.51 \text{ in.}^2)(2.23 \text{ in.}^2)} \\
 &= 2,320 \text{ ksi}
 \end{aligned}$$

$$\begin{aligned}
 H &= 1 - \frac{x_o^2 + y_o^2}{\bar{r}_o^2} \quad (\text{Spec. Eq. E4-8}) \\
 &= 1 - \frac{(0.668 \text{ in.})^2}{2.23 \text{ in.}^2} \\
 &= 0.800
 \end{aligned}$$

$$\begin{aligned}
 F_e &= \left( \frac{5,870 \text{ ksi} + 2,320 \text{ ksi}}{2(0.800)} \right) \left[ 1 - \sqrt{1 - \frac{4(5,870 \text{ ksi})(2,320 \text{ ksi})(0.800)}{(5,870 \text{ ksi} + 2,320 \text{ ksi})^2}} \right] \\
 &= 2,090 \text{ ksi}
 \end{aligned}$$

$$\begin{aligned}
 F_{cr} &= \left( 0.658^{\frac{F_y}{F_c}} \right) F_y \\
 &= \left( 0.658^{\frac{50 \text{ ksi}}{2,090 \text{ ksi}}} \right) (50 \text{ ksi}) \\
 &= 49.5 \text{ ksi}
 \end{aligned}$$

$$\begin{aligned}
 P_n &= F_{cr} A_{tee-crit} \\
 &= (49.5 \text{ ksi})(1.51 \text{ in.}^2) \\
 &= 74.7 \text{ kips}
 \end{aligned}$$

The available compressive strength of the tee is:

LRFD	ASD
From Table 4-10, $P_r = 49.0 \text{ kips}$ $P_u = \phi_c P_n$ $= 0.90(74.7 \text{ kips})$ $= 67.2 \text{ kips} > 49.0 \text{ kips} \quad \mathbf{o.k.}$	From Table 4-10, $P_r = 35.8 \text{ kips}$ $P_a = \frac{P_n}{\Omega_c}$ $= \frac{74.7 \text{ kips}}{1.67}$ $= 44.7 \text{ kips} > 35.8 \text{ kips} \quad \mathbf{o.k.}$

60 In the calculation of  $I_{x-comp}$  using Equation 4-38, the value of  $t_c$  should be 3.00 in.

64 Revise Equation 3-12 to:

$$T_o = M_r \left[ \frac{1 - \frac{(q)(X_i)}{T_1}}{d_{effect-noncomp}} \right]$$

88 Revise Equation 3-12 to:

$$T_o = M_r \left[ \frac{1 - \frac{(q)(X_i)}{T_1}}{d_{effect-noncomp}} \right]$$

102 Add  $d_{effect-noncomp}$  to the Symbols List after  $d_{effect-comp}$ :

$d_{effect-noncomp}$  Effective depth of noncomposite section, in. (mm)