

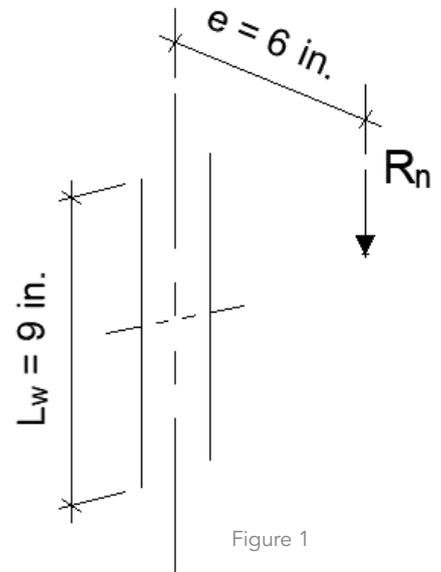
steel quiz

This month's Steel Quiz is based on guidance and equations provided on eccentrically loaded weld groups in Part 8 of the 15th Edition AISC *Steel Construction Manual*.

The question and answers for this month's Steel Quiz were contributed by Hamza Sekkak, a PhD student at the Illinois Institute of Technology. Thank you, Hamza!

Refer to Figure 1. Given that weld size, a , is $5/16$ in. and F_{EXX} is 70 ksi, solve for weld available strength, ϕR_n , using:

- 1 Table 8-4
- 2 Instantaneous center of rotation method
- 3 Elastic method
- 4 Plastic method



TURN PAGE FOR THE ANSWERS

steel quiz ANSWERS

- 1 **Using Table 8-4.** $k = 0$, because the force applied is out-of-plane with regard to the cross-sectional plane of the plate. From Table 8-4:

$$C = 1.84$$

$$\phi R_n = \phi C C_1 D I = (0.75)(1.84)(1.0)(5)(9) = 62.1 \text{ kips}$$

- 2 **Using the instantaneous center of rotation method.** Break half of the weld length into equal segments (see Figure 2).

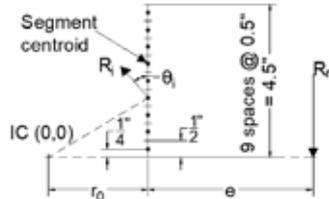


Figure 2

Select a trial location for the instantaneous center of rotation, r_0 . Compute coordinates of the centroids of the segments and their angles. Compute the deformations Δ_{mi} and Δ_{ui} using the following equations:

$$\Delta_{mi} = 0.209(\theta_i + 2)^{-0.32} w$$

$$\Delta_{ui} = 1.087(\theta_i + 6)^{-0.65} w \leq 0.17w$$

where θ_i is the segment angle in degrees and w is the weld size in in.

Compute Δ_i as follows:

$$\Delta_i = r_i \frac{\Delta_{ucr}}{r_{cri}} = r_i (0.0046)$$

Compute R_n , the resistance of each segment:

$$R_i = 0.60 F_{EXX} t_e l_w \left(1.0 + 0.50 \sin^{1.5} \theta_i \right) \left[\frac{\Delta_i}{\Delta_{mi}} \left(1.9 - 0.9 \frac{\Delta_i}{\Delta_{mi}} \right) \right]^{0.3}$$

| Vertical Segments | Length l_w (in.) | X (in.) | Y (in.) | r_i (in.) | R_i (kip) | $(R_i)_x$ (kip) | $(R_i)_y$ (kip) | $R_i r_i$ (kip-in.) |
|-------------------|--------------------|---------|---------|-------------|-------------|-----------------|-----------------|---------------------|
| 1 | 0.5 | 0.82 | 4.25 | 4.329 | 6.87 | 6.75 | 1.31 | 29.75 |
| 2 | 0.5 | 0.82 | 3.75 | 3.839 | 6.89 | 6.72 | 1.48 | 26.44 |
| 3 | 0.5 | 0.82 | 3.25 | 3.353 | 6.82 | 6.61 | 1.68 | 22.86 |
| 4 | 0.5 | 0.82 | 2.75 | 2.871 | 6.66 | 6.38 | 1.91 | 19.13 |
| 5 | 0.5 | 0.82 | 2.25 | 2.396 | 6.41 | 6.02 | 2.20 | 15.35 |
| 6 | 0.5 | 0.82 | 1.75 | 1.934 | 6.02 | 5.44 | 2.56 | 11.64 |
| 7 | 0.5 | 0.82 | 1.25 | 1.497 | 5.44 | 4.54 | 2.99 | 8.14 |
| 8 | 0.5 | 0.82 | 0.75 | 1.114 | 4.56 | 3.07 | 3.38 | 5.09 |
| 9 | 0.5 | 0.82 | 0.25 | 0.861 | 3.34 | 0.97 | 3.20 | 2.88 |
| | | | | | $\Sigma =$ | 46.51 | 20.71 | 141.27 |

Check rotational and force equilibrium until both values become the same ($r_0 = 0.824$ in.).

Rotational equilibrium:

$$R_n = \frac{2(\sum R_i r_i)}{e + r_0} = \frac{2(141.3)}{6.824} = 41.4 \text{ kips}$$

Force equilibrium:

$$R_n = 2 \sum (R_i)_y = 41.4 \text{ kips}$$

Finally:

$$\phi R_n = (0.75)(2 \text{ weld lines}) \times 41.4 \text{ kips} = 62.1 \text{ kips}$$

- 3 **Using the elastic method.**

The moment of inertia of the weld is:

$$I_x = 2 \left(\frac{9^3}{12} \right) = 121.50 \text{ in.}^3$$

Solve for the welding strength from the following:

$$\sqrt{\left(\frac{R_n}{2l} \right)^2 + \left(\frac{R_n e c}{I_p} \right)^2} = \left(0.707 \times \frac{5}{16} \text{ in.} \right) 0.6 (70 \text{ ksi})$$

$$R_n = 40.5 \text{ kips} \rightarrow \phi R_n = 30.4 \text{ kips}$$

- 4 **Using the plastic method.**

For one weld:

$$f_w = \sqrt{f_v^2 + (f_a + f_b)^2}$$

$$f_v = \frac{R_n}{l}, f_a = 0, f_b = \frac{4M}{l^2} = \frac{4R_n e}{l^2}$$

$$f_w = \left(\frac{5}{16} \text{ in.} \right) (0.707) (0.6) (70 \text{ ksi})$$

For two welds:

$$R_n = 58.7 \text{ kips} \rightarrow \phi R_n = 44.0 \text{ kips}$$

Notes: The Steel Quiz submitted by Hamza Sekkak did not account for the directional strength increase when applying the plastic method. Section J2.4.(b) of the AISC *Specification for Structural Steel Buildings* (ANSI/AISC 360) states: "For fillet welds, the available strength is permitted to be determined accounting for a directional strength increase of $(1.0 + 0.50 \sin^{1.5} \theta)$ if strain compatibility of the various weld elements is considered." Though not explicitly addressed in the use of the plastic method, strain compatibility will likely not be a problem for the condition shown. This is a matter of engineering judgment. If the directional strength increase is to be included, it can be done as follows:

$$\theta = \text{atan} \left(\frac{f_b}{f_v} \right) = \text{atan} \left(\frac{4e}{l} \right) = \text{atan} \left(\frac{4(6)}{9} \right) = 69.4^\circ$$

$$\left[1.0 + 0.50 \sin^{1.5} (69.4^\circ) \right] = 1.4$$

Accounting for the directional strength increase $\phi R_n = 63.9$ kips, this is within 3% of the strength predicted by the instantaneous center of rotation method, which explicitly considers strain compatibility of the various weld elements.

—Larry Muir, PE, AISC Director of Technical Assistance



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If you are interested in submitting one question or an entire quiz, contact AISC's Steel Solutions Center at 866.ASK.AISC or solutions@aisc.org.